GENERAL NOTES

1.1 Fabrication shall be in accordance with U.S.A.'s standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1". U.S.A.'s manufacturing procedures have been certified by:

Reference	Certification numbers
S.B.C.C.I.	#9011
I.C.B.O.	FA-327
MIAMI-DADE COUNTY	#1058
Houston	U.S.A. #135
Indiana (For Beam and Column)	299512
Indiana (For Rigid Frame)	299513
Indiana (For Straight Column)	299510
A.I.S.C. Certification For Metal Buildings	
Member MBMA	

1.2 MATERIALS

MATERIALS	ASTM DESIGNATION	MINIMUM YIELD (U. N.)
Wide Flange Shapes	A572 or A992	FY = 50 KSI
Other Rolled Mill Shapes	A36	FY = 36 KSI
Structural Steel Plate (Webs/Connections)	A1011, A935, A635	FY = 55 KSI
Structural Steel Plate (Flanges)	A529 OR A572	FY = 55 KSI
Cold Formed Light Gage Shapes	A1011SS	FY = 55 KSI
Cable Bracing	A475-78	Extra High Strength
Rod Bracing	A36	FY = 36 KSI
Roof and Wall Sheeting	A792	FY = 50 , 80 KSI
Machine Bolts	A307	FY = 36 KSI
High Strength Bolts (1/2"Ø to 1"Ø)	A325-N	FU = 120 KSI
High Strength Bolts (1 1/8"Ø to 1 1/2"Ø)	A325-N	FU = 105 KSI
Anchor Bolts (If supplied)	A36	FY = 36 KSI
Pipe	A53, GRADE B	FY = 35 KSI
H.S.S. TUBE	A500, GRADE B	FY = 46 KSI

1.3 PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification TT-P-664D and is U.S.A.'s Grey Oxide color. This paint is not intended for long term exposure to the elements. U.S.A. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or storage. U.S.A. shall not be responsible for any field applied paint and/or coatings. (Section 6.5 AISC Code of Standard Practice, 9th Edition)

1.4 GALVANIZED OR SPECIAL COATINGS:
See Contract Documents and ref. note on the top right hand of this page.

1.5 ALL BOLTS ARE 1/2"Ø x 0'-1" A307 EXCEPT:
a) Eave strut connection - 1/2"Ø x 0'-1 1/4" A307
b) Endwall rafter splice - 5/8"Ø x 0'-1 3/4" A325-N
c) Endwall column to rafter connection - 1/2"Ø x 0'-1 1/4" A325-N
d) Main frame connections - SEE CROSS SECTION

NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 A325 BOLT TIGHTENING REQUIREMENTS

All high strength bolts are A325-N unless specifically noted otherwise.

Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the 9th Edition AISC "Specification For Structural Joints Using ASTM A325 or A490 Bolts". A325-N bolts are supplied without washers unless otherwise noted on the drawings.

All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

1.7 CLOSURE STRIPS ARE FURNISHED FOR APPLICATION:
INSIDE- Under roof panels at eave
OUTSIDE - Between endwall panels and rake trim
- Under continuous ridge vent skirts

1.8 ERECTION NOTE:
All bracing, strapping, & bridging shown and provided by U.S.A. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 ERECTION AND UNLOADING NOT BY U.S.A.

1.10 SHORTAGES
Any claims or shortages by buyer must be made to U.S.A. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless U.S.A. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of U.S.A.

1.12 ERECTOR NOTE
Erect steel with piece marks corresponding to location of piece marks on erection drawings. Erect and plumb bays progressively to insure overall width and length is maintained.

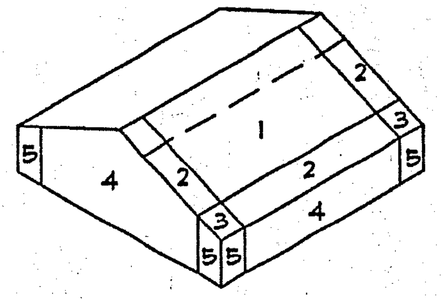
1.13 ACCESSORIES
Accessories are not wind rated.

1.14 ROOF SYSTEMS
The performance of roof systems supplied by USA is dependent on correct and proper erection and roof system installation by qualified roofing installers. USA is not responsible for non-performance due to improper or defective installation. USA does not certify erectors or roofing installers.

31	12/12/05
30	03/18/05
29	03/11/04
28	2/3/04
27	2/2/04
26	5/22/03

ALL FLAT FACED PANELS HAVE A TENDENCY TO OIL CAN.
USING HEAVIER GUAGES, NARROWER WIDTHS, EMBOSSING OR STIFFENER BEADS (WHEN AVAILABLE) CAN REDUCE OIL CANNING.
UNDER NO CIRCUMSTANCES IS OIL CANNING CAUSE FOR REJECTION

BASIC WIND PRESSURE: 18.86 psf
INTERNAL PRESSURE COEFFICIENT: 0.18
EDGE STRIP DIMENSION: 5.9 ft
COMPENENT AND CLADDING SUCTION:
ZONE 1 22.25 psf
ZONE 2 37.34 psf
ZONE 3 56.20 psf
ZONE 4 24.14 psf
ZONE 5 29.80 psf



U.S.A.

UNITED STRUCTURES OF AMERICA

DRAWING PACKAGE

JOB # 32827-306
BUYER : DEREK ENG.
END USE CUSTOMER : JEFFBOAT LLC.
BUYER ORDER # :
JOBSITE : JEFFERSONVILLE, IN COUNTY : CLARK
BASIC BUILDING : RF 58'- 10¼ x 222'- 4 x 46'-0 1 :12 (TWO BLDGS.)

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

CODE

(Code of Latest Issue to be Used Unless Otherwise Noted.)

Code Edition 2003

☐ USA Standard (MBMA)
☐ SBC ■ IBC
☐ UBC ☐ BOCA ☐ ASCE
EXPOSURE ☐ B ☐ C ☐ D
☐ OTHER ____
DEFLECTION: Per Building Code

LOADS

■ LL 20 PSF No Reduction
■ Collateral Load: 5 psf
■ Snow Load: Pg 15 psf Pf 9.45 psf Ce 0.9 Is 1.0 Ct 1.0
■ Wind Load 90 mph Iw 1.0 Exposure C
Internal Pressure Coefficient ±0.18 Design Wind Pressure 18.9 psf
■ Seismic Data: Use Group I Sds .261 Sd1 .183 Site Class D
Is 1.0 Ss .245 S1 .118 Cs .052
Seismic Design Category C
Seismic-force-resisting System: Ordinary steel moment frames R=4
Ordinary steel concentrically braced frames R=5
Design Base Shear V= 2.8 kips Equivalent Lateral Force Procedure
■ Crane Load: Capacity 10 tons Type TRE Class C
(See Cross Sections and/or calculations for loading)
☐ Mezzanine Load: DL____ psf LL____ psf CL____ psf
■ Other Loads: PLATFORM POINT LOADS

ROOF	■ 26GA ■ USA "PBR" ☐ 24GA ☐ ☐ 24GA SA "Guardian I" SSR ☐ 24GA SA "Guardian II" SSR ☐ ____ ☐ White ■ Aluminum Zinc ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000 Color ____ ☐ UL90-Const ____ ☐ Northern R ■ Wide Mastic Clip: ☐ Low ☐ High ☐ Fixed ☐ Floating ☐ Articulating Fasteners: ☐ #14 x 1" STS Member & Stitch ☐ #12 x 1¼" SDS Member Only W/ STS Stitch ■ #12 x 1¼" SDS Member with Laptek Stitch ■ Long Life Fasteners ☐ Stainless Steel Fasteners Thermal Blocks: ☐ By USA ☐ By Others Special Requirements ____
WALL	☐ 26GA ☐ USA "PBA" ☐ ____ ■ 24GA ■ USA "PBR" Color WHITE ☐ Kynar Unicote 3000 ■ Silicone Poly Unicote 2000 ☐ Long Life Fasteners ☐ Stainless Steel Fasteners Fasteners: ☐ #14 x 1" STS Member & Stitch ☐ #12 x 1¼" SDS Member Only W/ STS Stitch ■ #12 x 1¼" SDS Member with Laptek Stitch Special Requirements ____
TRIM	■ Standard W/Gutters & Downspout ☐ Simple Eave Trim ☐ Sculptured Eave Trim ■ Base Trim ■ MF01 ☐ #201 ☐ DP01 ☐ Base Closure Gutter WHITE Downspout WHITE Eave N/A Rake WHITE Corner WHITE Jamb & Head WHITE Base Trim WHITE ■ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000 Special Requirements ____
FASCIA PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ Trim Color ____ ☐ 24GA Special Requirements ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000
SOFFIT PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ Trim Color ____ ☐ 24GA Special Requirements ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000
BACK PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ Trim Color ____ ☐ 24GA Special Requirements ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000
PARTITION PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ Trim Color ____ ☐ 24GA Special Requirements ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000
LINER	☐ 26GA USA ☐ "PBA" ■ "PBR" ☐ ____ Color PWHITE Trim Color PWHITE ■ 24GA Special Requirements ____ ■ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000

QUALIFICATION OF HOT-DIP GALVANIZING PROCESS USED ON PRIMARY AND SECONDARY STEEL

The ability of a galvanized coating to meet its primary objective of providing corrosion protection should be the chief criteria in evaluating its overall appearance. Zinc coatings produced by the hot-dip galvanizing process are excellent corrosion-protection systems. When the coating becomes very thick or dull grey, the coating may not be applicable for architectural purposes.

Differences in the luster and color of galvanized coatings and the presence or absence of spangle has no affect on the coating performance. The well-known spangle effect found on galvanizing is simply a factor of primary crystallization. It is chiefly dependent upon the zinc bath chemistry, the rate of cooling, the method of pickling, the steel chemistry, and the thickness of the coating.

Additionally, handling techniques for galvanizing may require the use of chain slings, wire or other holding devices to immerse material into the galvanizing kettle. These handling devices may leave a mark on the galvanized item. These marks are not necessarily detrimental to the coating.

The galvanizing process is for corrosion protection. Inconsistencies in finish and handling marks are not cause for rejection of the product.

BUYER/END USE CUSTOMER RESPONSIBILITES

2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release U.S.A. to fabricate upon receiving such.

2.2 United Structures of America's (hereafter referred to as U.S.A.) standard specifications apply unless stipulated otherwise in the Contract Documents. U.S.A.'s design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between U.S.A.'s structural steel plans and plans for other trades, U.S.A.'s plans shall govern. (Section. 3 AISC Code of Standard Practices, 9th Edition)

2.4 Approval of U.S.A. drawings and calculations indicates that U.S.A. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the U.S.A.'s design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)

2.5 Once the BUYER/END USE CUSTOMER has signed U.S.A.'s Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.

2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by U.S.A. and U.S.A.'s steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or U.S.A.'s assumptions will govern (Section 4 and Commentary, AISC Code of Standard Practice, 9th Edition)

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that U.S.A.'s plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that U.S.A. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by U.S.A.

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with U.S.A.'s "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7 AISC Code of Standard Practice, 9th Edition)

2.9 United Structures of America is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete Unless otherwise provided in the Order Documents, United Structures of America does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Section A10 MBMA 1986 Low Rise Building Systems Manual)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to U.S.A. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (Section 7 AISC Code of Standard Practice, 9th Edition)

2.11 FIELD MODIFICATIONS - WARNING Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (Section 7 AISC Code of Standard Practice, 9th Edition)

The strength, stability, and safety of this building may be affected by cutting, drilling or removing any components of the building. No such modifications may be made without the written approval of United Structures of America, Inc.

Collateral loads or other components provided by others may be supported on this building in the designated locations if specifically included in the design loads. Unless the connections for such components have been designed and detailed by U.S.A. the customer is responsible to ensure appropriate connection details are utilized.

2.12 WARNING In no case should Aluminized Zinc steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Aluminized Zinc alloy coating when they are in contact with Aluminized Zinc steel panels. Even run-off from copper flashing, wiring, or tubing onto Aluminized Zinc should be avoided.

2.13 SAFETY COMMITMENT UNITED STRUCTURES of AMERICA has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of U.S.A.
It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure worker safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.
Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
Contractors/Erectors should be aware of OSHA regulations for Steel Erection. The following OSHA Regulations (Standards 29 CFR) are applicable to Steel Erection: OSHA Steel Erection Regulations, Part Number 1926, Subpart "R"

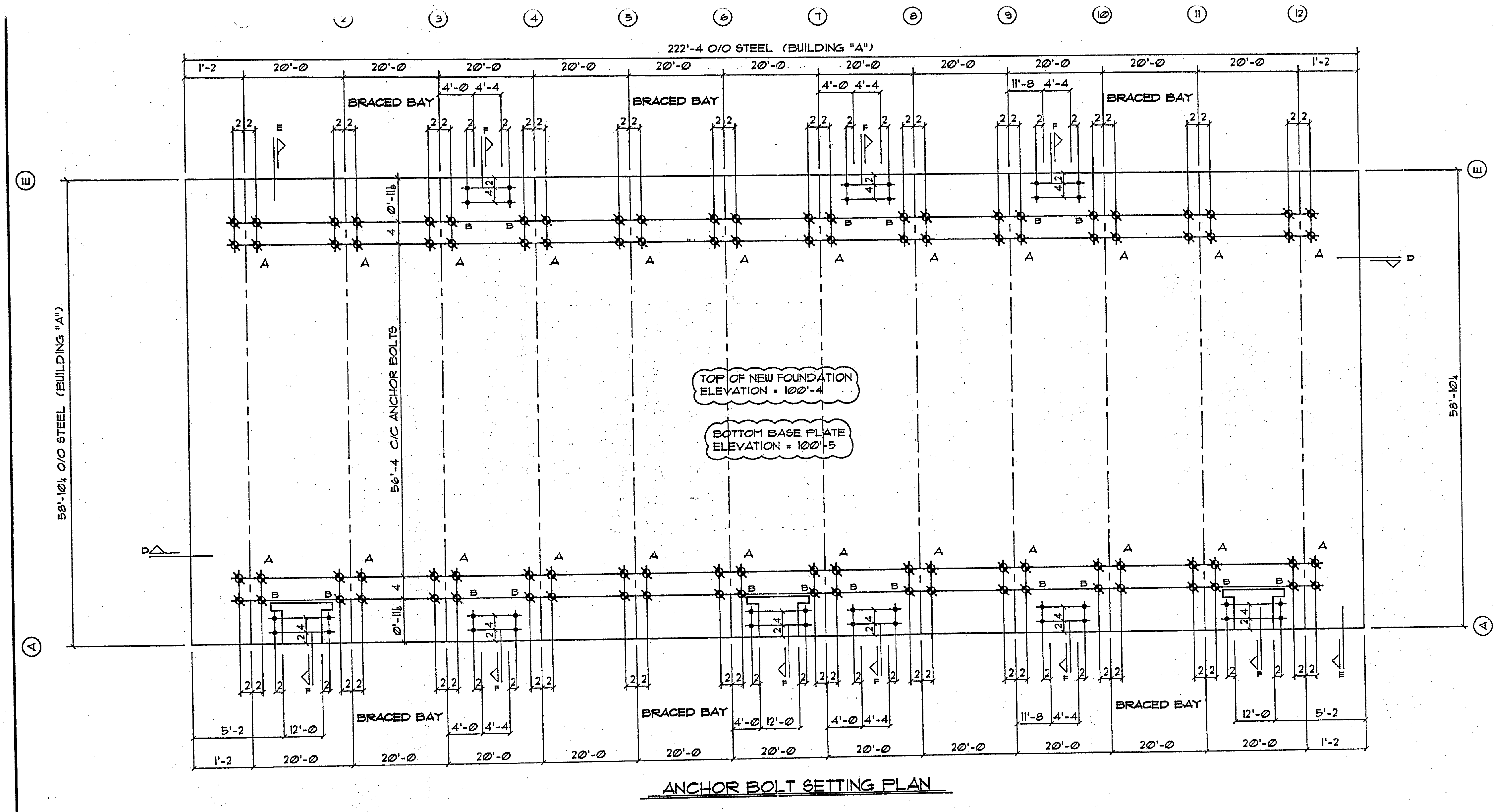
2.14 Manufacturer is not responsible for bodily injuries or material damages during unloading, handling or storage

2¼
25⅛
14
41 3/8

0	CONSTRUCTION	CM 4/26/06
A/P1	APPVL/PERMIT	4/18/06
Job Number : 32827-306		
SHEET L1 of 1		

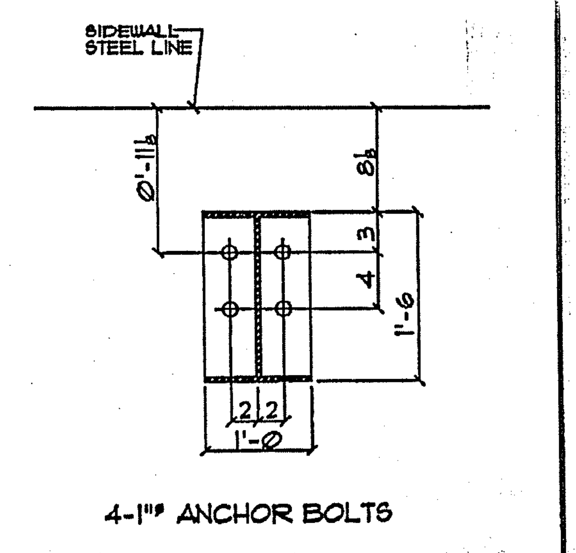
FOR CONSTRUCTION

George Childers
Line 1 Building



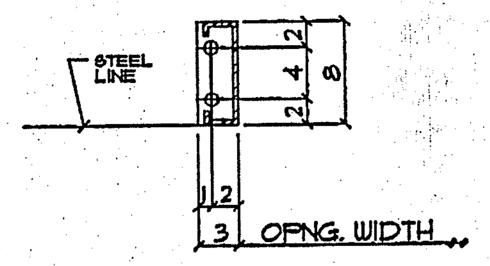
ANCHOR BOLT SETTING PLAN

NOTE! SEE SHEET F3 FOR REACTIONS.



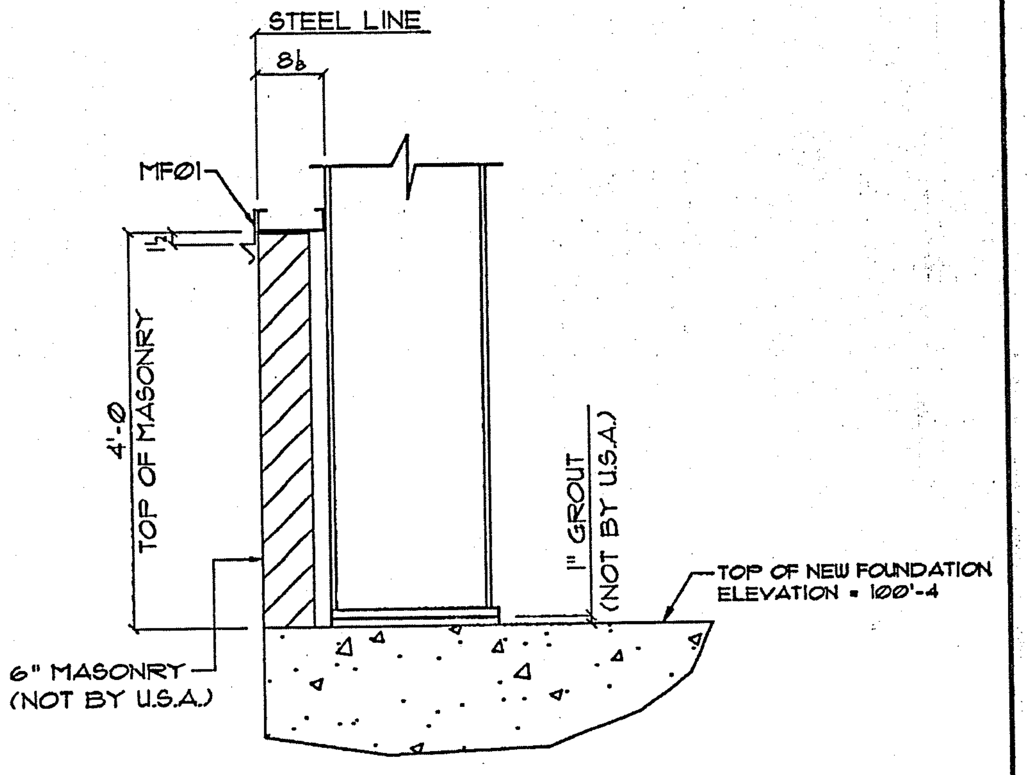
4-1"ᶲ ANCHOR BOLTS

BASE TYPE "A"



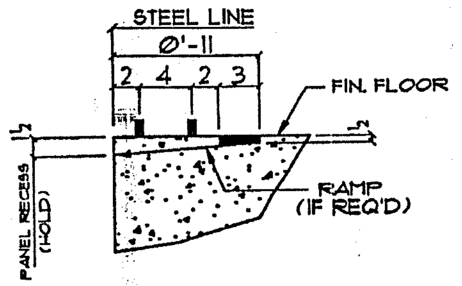
2-¾"ᶲ ANCHOR BOLTS

BASE TYPE "B"

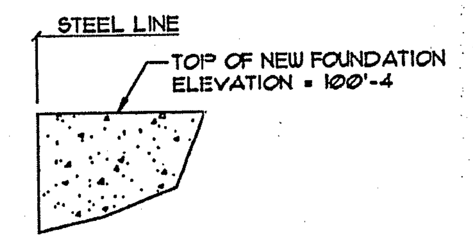


STEEL LINE
8⅛
MF01
1½
4'-0 TOP OF MASONRY
1" GROUT (NOT BY U.S.A.)
TOP OF NEW FOUNDATION ELEVATION = 100'-4
6" MASONRY (NOT BY U.S.A.)

SECTION "E"



SECTION "F"



SECTION "D"

GENERAL NOTES:

① The Anchor Bolt Details shown on this drawing locate the Anchor Bolts in reference to both the Building Steel Line and the outside of U.S.A.'s SUGGESTED panel recess of 1-1/2".

② United Structures of America is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing, and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete.
Unless otherwise provided in the Order Documents, United Structures of America does not design and is not responsible for the design, material, and construction of the foundation or foundation embedments. The End Use Customer should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures.

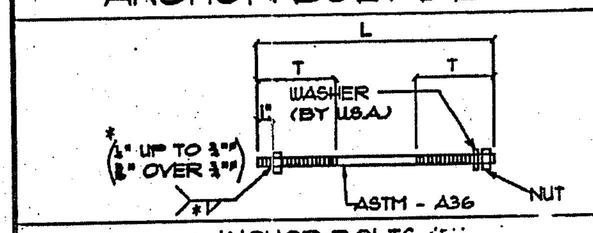
③ Botton of all Base Plates are at the same elevation. (UNLESS NOTED)

④ NOTE: SHEAR ANGLES AND/OR ANCHOR TIES ARE NOT FURNISHED BY U.S.A.

NOTE:
Anchor bolt lengths shown below are USA Standard Stock Lengths only. It is the responsibility of the customer to determine the proper anchor bolt embedment length.
Ref. General Note #2 of this drawings.
(If Anchor Bolts are supplied by U.S.A.,
Customer must specify required length to USA).

NOTE:
ANCHOR BOLT PROJECTION ARE FROM TOP OF CONCRETE.

ANCHOR BOLT DETAIL



ANCHOR BOLTS (...)

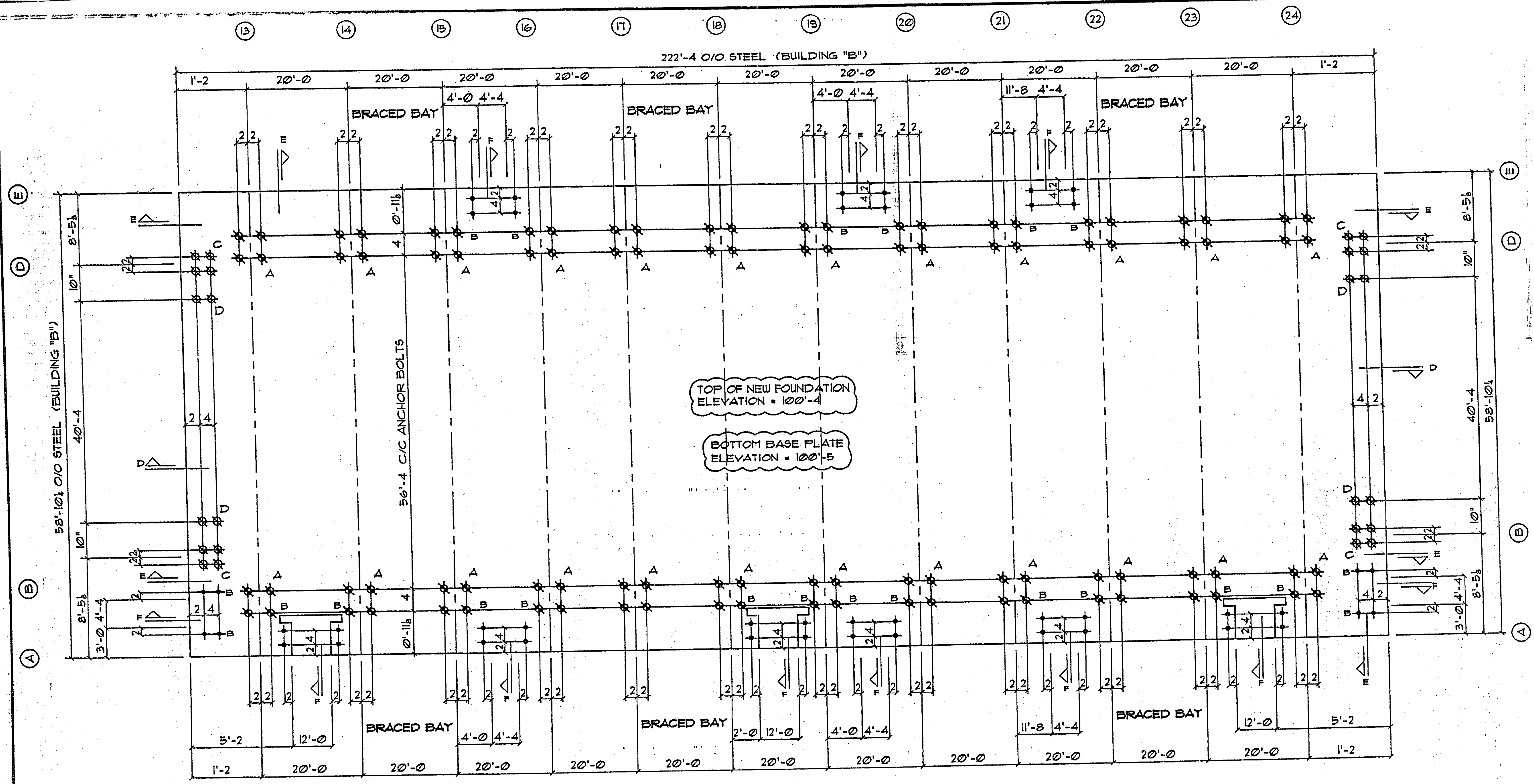
PcMk	SYMBOL	DIA.	QTY.	L	T	PROJ.	ISSUE	REV. DESCRIPTION	DATE	BY / DSN
A4	✦	1/2"	36	A307 M.B. x 0'-6" W/ NUT	1 1/4"	1"	A/PI	APPV'L./PERMIT	4/18/06	CM
A5	⊕	5/8"		1'-2"	2"	2"	0	CONSTRUCTION	5/17/06	CM
A6	⊕	3/4"		1'-5"	3"	3"				
A7	✱	7/8"		1'-8"	3"	3"				
A8	✱	1"	96	2'-2"	3"	3"				
A10	✱	1 1/4"		2'-4"	4"	4"				
A12	⊕	1 1/2"		2'-6"	4"	4"				

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
1912 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION: ANCHOR BOLT SETTING PLAN			
CUSTOMER: DEREK ENG.			
LOCATION: JEFFERSONVILLE, IN			
BUYER NO.: JEFFBOAT LLC.			
DRAWING BY JY	DATE 4/06	CHECK BY CM	DATE 5/17/06
JOB # 32827-306	SCALE N.T.S.	DRAWING # F1 of 3	ISSUE 0

32827-FOLDWG

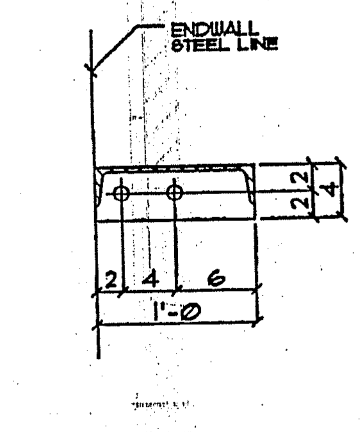
FOR CONSTRUCTION



13
14
15
16
17
18
19
20
21
22
23
24
222'-4 O/O STEEL (BUILDING "B")
1'-2
20'-0
BRACED BAY
4'-0 4'-4
11'-8 4'-4
E
D
B
A
58'-10½ O/O STEEL (BUILDING "B")
56'-4 C/C ANCHOR BOLTS
8'-5½
10"
40'-4
3'-0
4'-4
0'-11½
TOP OF NEW FOUNDATION ELEVATION = 100'-4
BOTTOM BASE PLATE ELEVATION = 100'-5
5'-2
12'-0
2'-0

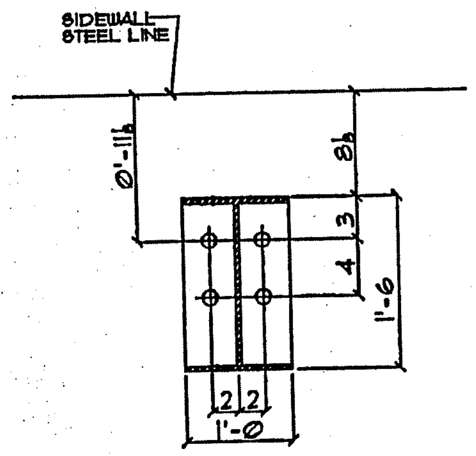
ANCHOR BOLT SETTING PLAN

NOTE! SEE SHEET F3 FOR REACTIONS.



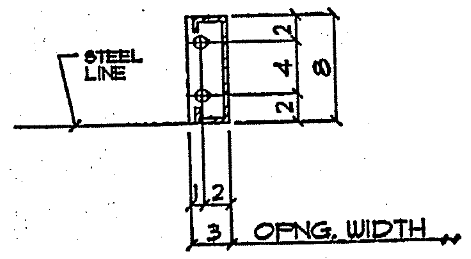
2-¾"ø ANCHOR BOLTS

BASE TYPE "D"



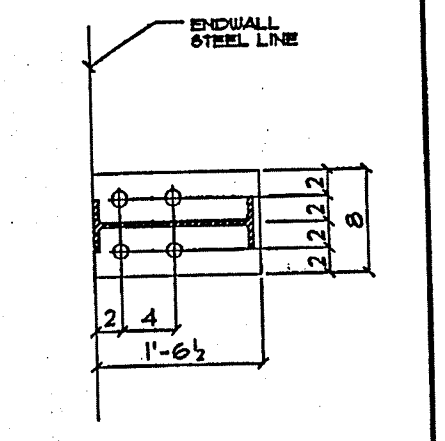
4-1"ø ANCHOR BOLTS

BASE TYPE "A"



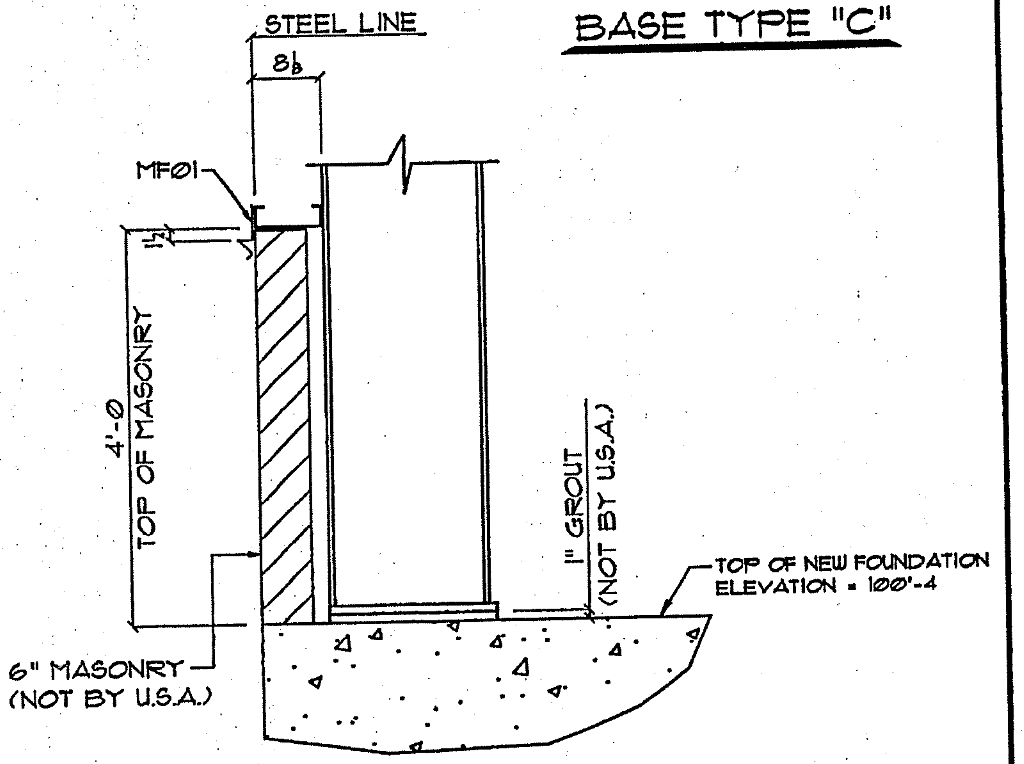
2-½"ø ANCHOR BOLTS

BASE TYPE "B"



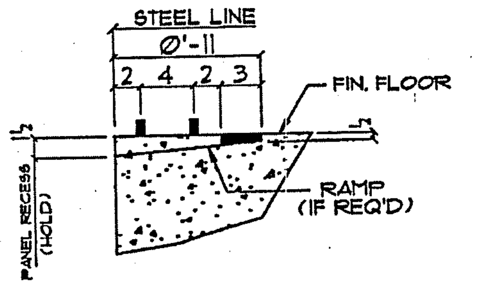
4-¾"ø ANCHOR BOLTS

BASE TYPE "C"

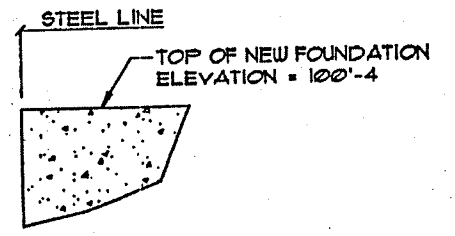


STEEL LINE
8½
MF01
TOP OF MASONRY
4'-0
1" GROUT (NOT BY U.S.A.)
TOP OF NEW FOUNDATION ELEVATION = 100'-4
6" MASONRY (NOT BY U.S.A.)

SECTION "E"



SECTION "F"



SECTION "D"

GENERAL NOTES:

①The Anchor Bolt Details shown on this drawing locate the Anchor Bolts in reference to both the Building Steel Line and the outside of U.S.A.'s SUGGESTED panel recess of 1-1/2".

②United Structures of America is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing, and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete.
Unless otherwise provided in the Order Documents, United Structures of America does not design and is not responsible for the design, material, and construction of the foundation or foundation embedments. The End Use Customer should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures.

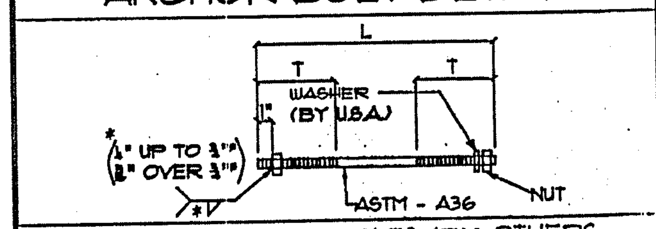
③Bottom of all Base Plates are at the same elevation. (UNLESS NOTED)

④NOTE: SHEAR ANGLES AND/OR ANCHOR TIES ARE NOT FURNISHED BY U.S.A.

NOTE:
Anchor bolt lengths shown below are USA Standard Stock Lengths only. It is the responsibility of the customer to determine the proper anchor bolt embedment length.
Ref. General Note #2 of this drawings.
(If Anchor Bolts are supplied by U.S.A.
Customer must specify required length to USA).

NOTE:
ANCHOR BOLT PROJECTION ARE FROM TOP OF CONCRETE.

ANCHOR BOLT DETAIL



ANCHOR BOLTS (BY OTHERS

PcMk	SYMBOL	DIA.	QTY.	L		T	PROJ.
A4	✦	1/2"	44	A307 M.B. x 0'-6" W/ NUT		1 1/4"	1"
A5	⊕	5/8"		1'-2"		2"	2"
A6	⊛	3/4"	24	1'-5"		3"	3"
A7	❋	7/8"		1'-8"		3"	3"
A8	⊗	1"	96	2'-2"		3"	3"
A10	✺	1 1/4"		2'-4"		4"	4"
	⊕	1 1/2"		2'-6"		4"	4"

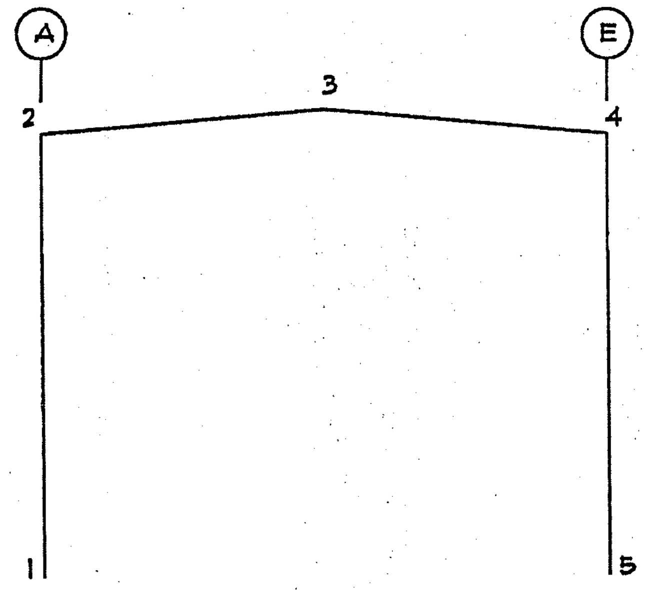
ISSUE	REV. DESCRIPTION	DATE	BY / DSN
A/P1	APPV'L/PERMIT	4/18/06	CM
0	CONSTRUCTION	5/17/06	CM

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
1912 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

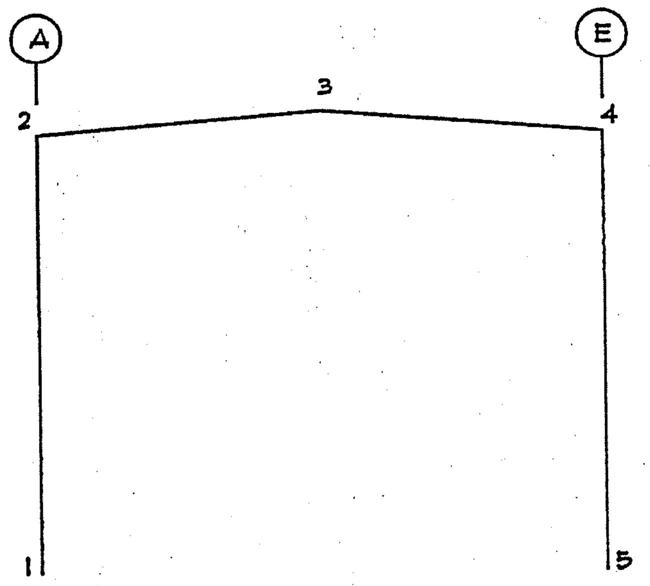
DESCRIPTION: ANCHOR BOLT SETTING PLAN
CUSTOMER: DEREK ENG.
LOCATION: JEFFERSONVILLE, IN
BUYER NO.: JEFFBOAT LLC.

DRAWING BY	DATE	CHECK BY	DATE
JY	4/06	CM	5/17/06
JOB # 32827-306	SCALE N.T.S.	DRAWING # F2 of	ISSUE 0

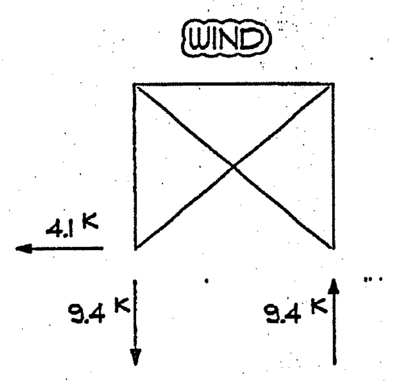
32827-F02.DWG



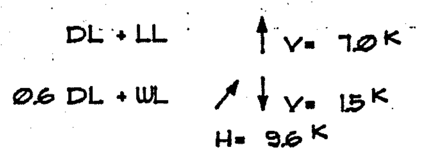
REACTION FOR LINES 1 THRU 12 (BUILDING "A")					
NO.	LOAD CONDITIONS	V1	H1	V5	H5
1	DEAD + LIVE LOAD	22	4	22	-4
2	0.6DL + LWP	-9	-6	2	-6
3	0.6DL + LWS	-5	-9	6	-4
4	0.6DL + RWP	2	6	-9	6
5	0.6DL + RWS	6	4	-5	9
6	0.6DL + LEP	-6	4	-6	-4
7	0.6DL + REP	-5	4	-6	-4
8	DL + SUL + CL	18	3	14	-3
9	DL + SUR + CL	14	3	18	-3
10	DL + 0.75(LWS + SUL)	7	-5	11	-4
11	DL + 0.75(RWS + SUL)	15	4	4	5
12	DL + 0.75(LWS + SUR)	4	-5	15	-4
13	DL + 0.75(RWS + SUR)	11	4	7	5
14	DL + LL + MEZZ	79	11	79	-11
15	DL + .75SNOW + CL + .75ML	95	15	58	-3
16	DL + .75SNOW + CR + .75ML	58	3	95	-15
17	DL + 0.5LWP + CR	2	-8	44	-11
18	DL + 0.5LWS + CR	4	-9	46	-10
19	DL + 0.5RWP + CL	44	11	2	8
20	DL + 0.5RWS + CL	46	10	4	9



REACTION FOR LINES 13 THRU 24 (BUILDING "B")					
NO.	LOAD CONDITIONS	V1	H1	V5	H5
1	DEAD + LIVE LOAD	22	4	22	-4
2	0.6DL + LWP	-9	-6	2	-6
3	0.6DL + LWS	-5	-9	6	-4
4	0.6DL + RWP	2	6	-9	6
5	0.6DL + RWS	6	4	-5	9
6	0.6DL + LEP	-6	4	-6	-4
7	0.6DL + REP	-5	4	-6	-4
8	DL + SUL + CL	18	3	14	-3
9	DL + SUR + CL	14	3	18	-3
10	DL + 0.75(LWS + SUL)	7	-5	11	-4
11	DL + 0.75(RWS + SUL)	15	4	4	5
12	DL + 0.75(LWS + SUR)	4	-5	15	-4
13	DL + 0.75(RWS + SUR)	11	4	7	5
14	DL + LL + MEZZ	79	11	79	-11
15	DL + .75SNOW + CL + .75ML	95	15	58	-3
16	DL + .75SNOW + CR + .75ML	58	3	95	-15
17	DL + 0.5LWP + CR	2	-8	44	-11
18	DL + 0.5LWS + CR	4	-9	46	-10
19	DL + 0.5RWP + CL	44	11	2	8
20	DL + 0.5RWS + CL	46	10	4	9

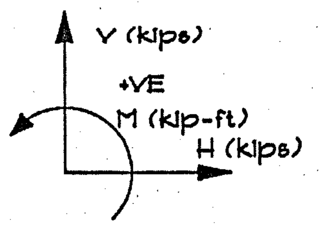


BRACED BAY REACTION
BUILDING "A" & "B"



MAX. E.W. COLUMN REACTION AT END FRAME
BUILDING "B"

Column Reactions Sheet
DL: Dead Load
LL: Roof Live Load
MLL: Mezzanine Live Load
LWP: Wind from Left w/ interior pressure
LWS: Wind from Left w/ interior suction
RWP: Wind from Right w/ interior pressure
RWS: Wind from Right w/ interior suction
LEP: Wind parallel to ridge (left orientation)
REP: Wind parallel to ridge (right orientation)
WLL: Wind from Left
WLR: Wind from Right
SBA: Balanced Snow Load
SUL: Unbalanced Snow on Left
SUR: Unbalanced Snow on Right
SBD: Uniform Snow with Drift Conditions
SBS: Uniform Snow with Sliding Snow
CRI: Crane Loads - Case 1, 2, etc.
EQL: Seismic Loads from Left
EQR: Seismic Loads from Right



SIGN CONVENTION

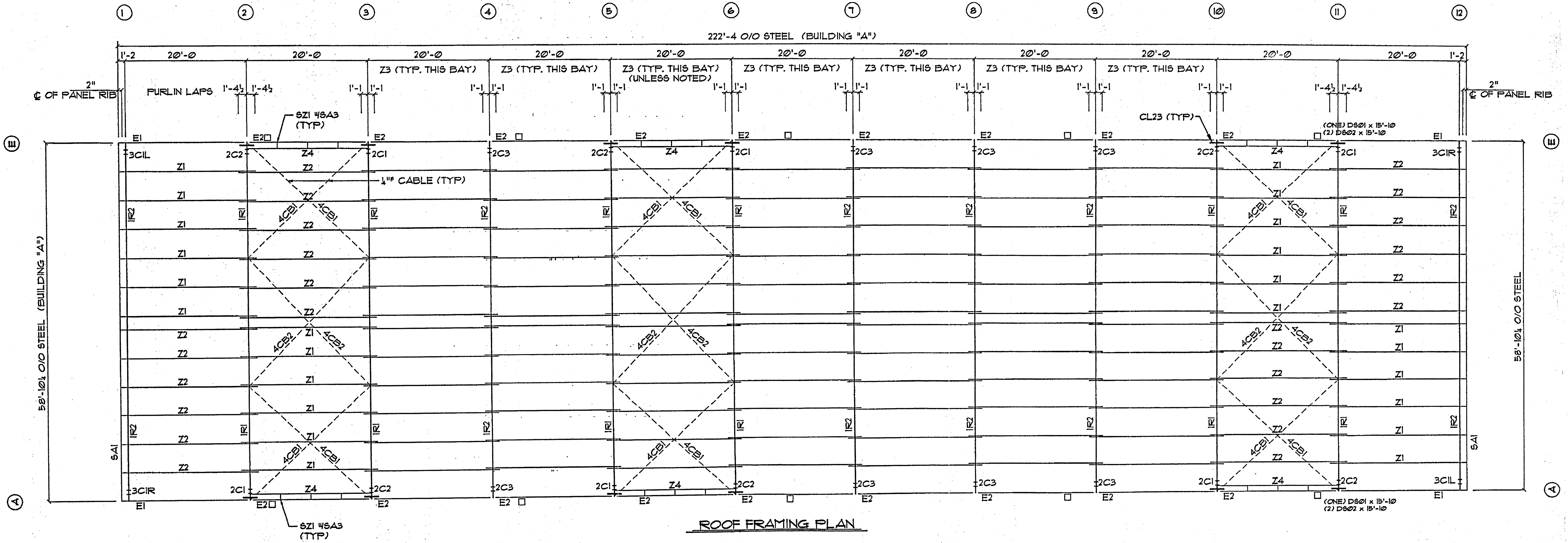
ISSUE	REV. DESCRIPTION	DATE	BY / DSN
A/P1	APPV'L/PERMIT	4/18/06	CM
0	CONSTRUCTION	5/17/06	CM

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
1912 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION: COLUMN REACTIONS
CUSTOMER: DEREK ENG.
LOCATION: JEFFERSONVILLE, IN
BUYER NO.: JEFFBOAT LLC.

DRAWING BY	DATE	CHECK BY	DATE
JY	4/06	CM	5/17/06
JOB # 32827-306	SCALE N.T.S.	DRAWING # F3 of 3	ISSUE 0

FOR CONSTRUCTION



ROOF FRAMING PLAN

PANELS	**TRIM**	**ACCESSORIES**					
		MK.	QTY.	DESCRIPTION	MK.	QTY.	DESCRIPTION
ROOF-26GA/PBR/AZINC	GUTTER-24GA/WHTE						
WALL-24GA/PBR/WHTE	DOWNSPOUT-24GA/WHTE	A	8	12-0 X 12-0 FRAMED OPNG. (FOR OHD)			
	CORNER-24GA/WHTE	B	14	4'-4 X 7'-2 FRAMED OPNG. (FOR DOORS)			
	JAMB/HEAD-24GA/WHTE	C	LOT	880 LINEAL FT. X 27-6 (24GA/PBR/P.WHITE) LINER			
	BASE-24GA/WHTE	D	2	40'-0 X 30'-0 FRAMED OPNG. (FOR CYLINDER DOORS)			
	RAKE-24GA/WHTE						

GENERAL NOTE:
All Bolts Are 1/2"Ø x 0'-1 (A307) EXCEPT:
1. Eave Strut Connection - 1/2"Ø x 0'-1 1/4 (A307)
2. Main Frame Connections - SEE CROSS-SECTION

Closure Strips Are Furnished For Application
INSIDE - Under Roof Panel At Eave
OUTSIDE - Between Endwall Panels And Rake Trim

Gutter Straps (SG01) To Be Installed At 4'-0 On Center For EG01 Gutter.

ERECTOR NOTE

Erect steel with piece marks corresponding to location of piece marks on erection drawings. Erect and plumb bays progressively to insure overall width and length is maintained.

ISSUE	REV. DESCRIPTION	DATE	BY / DSN
A/P1	APPV'L./PERMIT	4/18/06	CM
0	CONSTRUCTION	5/26/06	CM

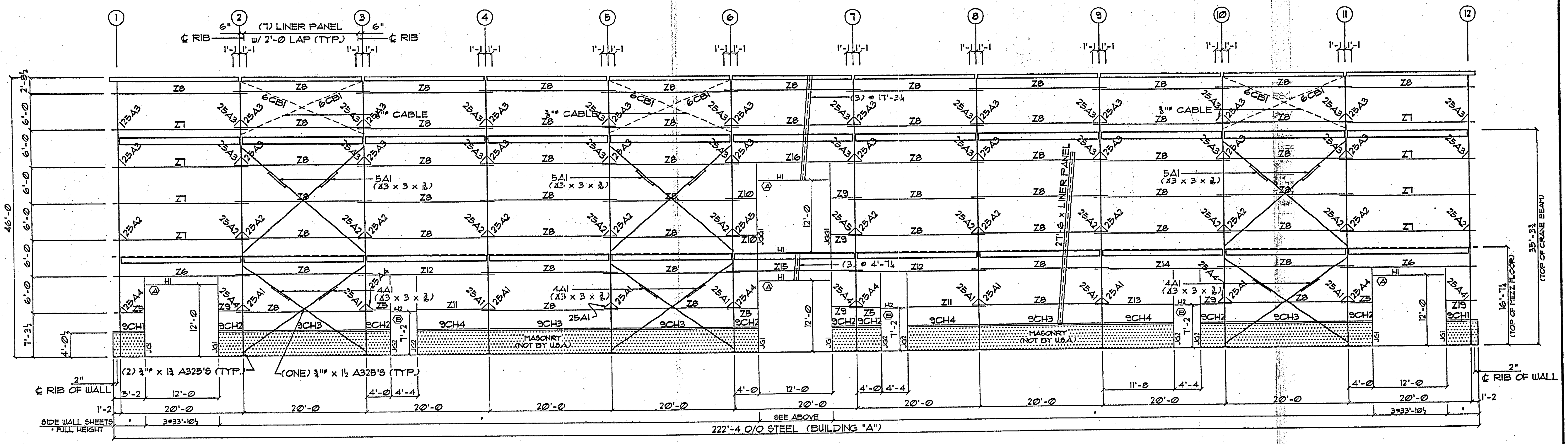
U.S.A.
UNITED STRUCTURES OF AMERICA, INC.

DESCRIPTION: ERECTION PLAN
CUSTOMER: DEREK ENG.
LOCATION: JEFFERSONVILLE, IN
BUYER NO.: JEFFBOAT LLC.

DRAWING BY	DATE	CHECK BY	DATE
JY	4/06	CM	5/26/06
JOB # 32827-306	SCALE N.T.S.	DRAWING # E1 of 17	ISSUE 0

32827-E01.DWG

FOR CONSTRUCTION



ELEVATION @ LINE "A"

ISSUE	REV. DESCRIPTION	DATE	BY DSN
A/P1	APPV'L./PERMIT	4/18/06	CM
0	CONSTRUCTION	5/26/06	CM

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
1512 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION: ERECTION PLAN
CUSTOMER: DEREK ENG.
LOCATION: JEFFERSONVILLE, IN
BUYER NO.: JEFFBOAT LLC.

DRAWING BY	DATE	CHECK BY	DATE
JY	4/06	CM	5/26/06
JOB # 32827-306	SCALE	DRAWING #	ISSUE 0

FOR CONSTRUCTION

32827-E01.DWG